Proposed Generally Applicable Fragility curve parameters for 154kV Transmission Towers under Extreme Wind Load

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1. Introduction

Extreme wind events driven by climate change pose significant threats to the stability of power grids, and structural damage to transmission towers—one of their key components—can lead to localized outages and even large-scale blackouts [1]. For instance, Typhoon Maemi in 2003 caused the collapse of 12 transmission towers in South Korea, resulting in power outages, while Typhoon Mujigae in China in 2014 damaged 206 towers, leading to widespread blackouts [2,3]. These cases highlight the necessity of conducting fragility analyses transmission towers in system-level safety assessments of power grids; however, such analyses are constrained by substantial time and cost requirements. To address this challenge, the present study proposes generally applicable fragility parameters for 154 kV transmission towers, which are the most widely deployed in the Korean power grid.

2. Nonlinear Pushover Analysis

2.1 154kV Tower Model and Wind Load Calculation

The representative 154kV transmission tower model was adopted from the study of Kim et al. [4] and corresponds to the most widely used tower type in the main transmission network of South Korea. The front and side elevations of the tower are presented in Fig. 1, where the tower was divided into panels according to IEC 60826 in order to define the wind load application zones [5]. For wind load estimation, the procedures specified in IEC 60826 were followed, and three equations were employed to calculate the wind loads acting on both the transmission tower and the conductors.

$$q_0 = 0.5\tau \mu (K_R V_{RB})^2 \tag{1}$$

$$A_{t} = q_{0}(1 + 0.2\sin^{2}2\theta)(S_{t1}C_{xt}\cos^{2}\theta +_{t2}C_{xt}\sin^{2}\theta)G_{t}$$
(2)
$$A_{c} = q_{0}C_{xc}G_{c}G_{L}dL\sin^{2}\Omega$$
(3)

Here, q_0 denotes the dynamic reference wind pressure, while A_t and A_c represent the wind loads acting on the external panels of the tower and on the transmission lines, respectively. The parameters τ and μ correspond to the air density correction factor and the air mass per unit volume, for which conservative values of 1.19 and 1.225 kg/m^3 were adopted. K_R denotes the roughness factor, and V_{RB} is the 1-minute mean wind speed measured at a height of 10 m above ground in terrain category B. S_t represents the projected area of the tower members within each panel, and C_{xt} refers to the drag coefficients in the x- and y-directions. G_t is the combined wind factor, while C_{xc} , the drag coefficient for conductors, was conservatively set to 1. The conductor diameter (d) was taken as 28 mm, G_L denotes the span factor, and Ω is defined as 90° - θ . The θ between the transmission line and the wind direction was assumed to be perpendicular for wind load estimation.

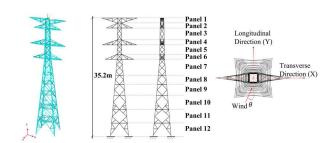


Fig. 1. 154kV transmission tower model.

2.2 Aleatory and Epistemic Uncertainties Variables

To derive generally applicable fragility parameters, both epistemic and aleatory uncertainty variables were represented using probability distributions, as summarized in Table I. A total of 100 sample models were generated through Latin Hypercube Sampling (LHS). For epistemic uncertainty variables, the yield strength (f_y) , ultimate strength (f_u) , elastic modulus (E), Poisson's ratio (v), and ultimate strain (ε_u) were considered, each assumed to follow a lognormal distribution. The mean values of these variables were obtained from the Korean Design Standard (KDS 14 31

05), while their coefficients of variation (COV) were determined based on the *Probabilistic Model Code* published by the Joint Committee on Structural Safety (JCSS) [6,7].

Among the aleatory uncertainty variables, the wind direction was modeled using a uniform distribution ranging from 0° to 90°. This accounts for the fact that, in addition to straight-winds, non-uniform wind conditions such as typhoons can impose loads on transmission towers from various wind direction. Furthermore, the span length between towers was incorporated as an input variable, extracted from spatial data provided by OpenStreetMap [8].

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Tuble 1: Considered alcutory and epistemic variables.								
Material parameter	Probability distribution	Mean value	Coefficient of variation					
Yield Stress			0.07					
Ultimate Stress		410 MPa	0.04					
Poisson's ratio	Lognormal	0.3	0.03					
Elastic modulus		210,000 MPa	0.03					
Ultimate strain		0.02	0.06					
Wind direction	Uniform	367.7m	0.58					
Span length	Fisk	512.3m	0.42					

2.3 Results of Nonlinear Pushover Analysis

For the nonlinear analysis of the 154 kV transmission tower, numerical modeling was performed using PyMAPDL. Each structural member was modeled with BEAM188 elements, and the material properties were defined as SS275 and SS410. A bilinear steel model was employed to capture the inelastic behavior. The wind speed range was set from 10 m/s to 100 m/s, with increments of 5 m/s, and the analyses were conducted accordingly. To determine failure, the displacement limit was defined as 1.5% of the tower height, following precedents in prior studies [9,10]. For the 154 kV tower, this corresponded to a failure displacement of 528 mm.

Figure 3 presents the nonlinear analysis results for the 154 kV transmission tower model. The red solid line represents the average response across all simulations and indicates the average capacity curve derived through curve fitting. The analysis results showed that at a top displacement of approximately 0.6 m, the corresponding average base shear was about 580 kN.

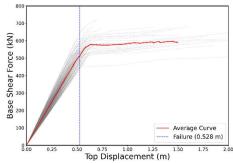


Fig. 3. Pushover curve for 154kv tower

3. Fragility analysis for 154kV tower

Based on the nonlinear analysis results, a fragility analysis of the 154 kV transmission tower model was conducted. The fragility curves, presented in Figure 5, were derived for wind direction angles in 30° intervals. For the range of 0°–30°, the parameters were obtained as $V_m = 51.63$ m and $\beta = 0.109$. For 30°–60°, the results were $V_m = 50.77$ m/s and $\beta = 0.130$. In the 60°–90° range, V_m slightly increased to 50.97 m/s, while β decreased to 0.111. For the overall range of 0°–90°, the fragility parameters were $V_m = 51.13$ m/s and $\beta = 0.117$, indicating relatively consistent behavior across all wind directions.

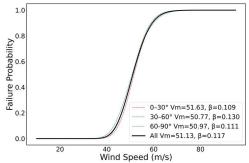


Fig. 4. Fragility curve for 154kV transmission tower

4. Proposed Fragility Curve Parameters and Discussion

The consideration of uncertainty factors is essential to propose generally applicable fragility curves for 154 kV transmission towers under extreme wind conditions. Such consideration enhances the reliability of fragility analyses and power grid safety assessments. In this study, two additional sources of uncertainty were incorporated, and the final combined logarithmic standard deviation, $\beta_{Proposed}$, was formulated as the sum of variances, as presented in Equation (4).

$$\beta_{proposed}^2 = \beta_{Analysis}^2 + \beta_{WM}^2 + \beta_{MDL}^2$$
 (4)

Here, $\beta_{Proposed}$ refers to the logarithmic standard deviation derived from the fragility analysis results presented in Chapter 3 (Fig. 4). β_{WM} represents modeling uncertainty, and β_{MDL} denotes the uncertainty

associated with the representative 154 kV transmission tower model.

First, β_{WM} was taken as 0.05, following the coefficient proposed by Ellingwood (1978) [11]. This coefficient accounts for modeling uncertainties arising during structural analysis, reflecting the possibility that the numerical model may not fully capture the actual structural capacity of the transmission tower.

Second, β_{MDL} was determined with reference to the *Quality Rating of Index Archetype Models* provided in FEMA P695 [12]. This table quantifies uncertainties associated with representative models by classifying them into High, Medium, and Low categories based on two evaluation criteria, and assigning corresponding values of β_{MDL} . In this study, a value of 0.35, corresponding to the medium category, was adopted.

Finally, as summarized in Table II, generally applicable fragility parameters are proposed for 154 kV transmission towers. The proposed parameters are based on 10-minute mean wind speeds but can be converted to 3-second gust wind speeds by applying the appropriate gust factor to V_m .

Table II. Proposed fragility curve variables for 154kV tower

		V_m (m/s)	B Proposed
Proposed variables	154kV	50.77	0.38

5. Conclusions

In this study, nonlinear analysis and fragility assessment were performed for 154 kV transmission towers commonly used in the Korean power grid, yielding the median capacity (V_m) and logarithmic standard deviation (β). In addition, modeling uncertainty and the uncertainty associated with the representative 154 kV tower model were further incorporated. Ultimately, generally applicable fragility parameters were proposed for 154 kV transmission towers. These parameters can provide useful guidance for practical decision-making in system-level safety assessments of power grids.

6. ACKOWLEDGEMENT

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REFERENCES

- [1] Ministry of Environment, Korea Meteorological Administration, Korea Environment Institute, Korean Climate Change Assessment Report 2020: Impacts and Adaptation Summary for Policymakers. Ministry of Environment, Republic of Korea, 2020. (In Korean)
- [2] National Disaster Management Research Institute (NDMI), Field Survey Report of Damages Caused by Typhoon Maemi

- in 2003. Ministry of the Interior and Safety, pp. 1–292, 2003. (In Korean)
- [3] L. An, J. Wu, Z., Zhang, and R., Zhang, Failure Analysis of a Lattice Transmission Tower Collapse due to the Super Typhoon Rammasun in July 2014 in Hainan Province, China. Journal of Wind Engineering & Industrial Aerodynamics, Vol.182, pp.295–307, 2018.
- [4] G., Kim, G., Chung, Y., Choun, Y., and S., Chang, Study on the High Wind Fragility Assessment for 154kV Transmission Tower using LHS. Proceedings of the Transactions of the Korean Nuclear Society Spring Meeting, May 9–10, 2024, Jeju, Korea.
- [5] International Electrotechnical Commission (IEC), IEC 60826:2017 Overhead Transmission Lines Design Criteria, Edition 4.0, Geneva, Switzerland, 2017.
- [6] Ministry of Land, Infrastructure and Transport (MOLIT), KDS 14 31 05: General Provisions for Steel Structure Design (Load and Resistance Factor Design method). Korea Design Standards, Republic of Korea, 2024. (In Korean)
- [7] Joint Committee on Structural Safety (JCSS), Probabilistic Model Code, Part 3: Resistance Models Structural Steel. JCSS, Zurich, Switzerland, 2001.
- [8] OpenStreetMap, 2024, https://www.openstreetmap.org.
- [9] Y., Sang, M., Sahraei-Ardakani, J., Xue, and G., Ou, Comparing a New Power System Preventive Operation Method with a Conventional Industry Practice during Hurricanes. 2019 North-American Power Symposium, 13-15 Oct. Wichita, KS, USA, pp. 1-6, 2019.
- [10] Y., Sang, M., Sahraei-Ardakani, J., Xue, and G., Ou, An Integrated Preventive Operation Framework for Power Systems during Hurricanes. IEEE Systems Journal, Vol.14, pp.3245-3255, 2020.
- [11] B., Ellingwood, Reliability Basis of Load and Resistance Factors for Reinforced Concrete Design. NBS Building Science Series 110, National Bureau of Standards, U.S. Department of Commerce, Washington, D.C., 1978.
- [12] FEMA, Quantification of Building Seismic Performance Factors (FEMA P695). Federal Emergency Management Agency, Washington D.C, 2009.
- [13] J., Woo, J., Jung, S., Kwag, and S., Eem, Proposal of General Fragility Curve Variables for Korean Transmission Towers on Wind Load, Submitted for publication, 2025